Strength of Materials Supplement for Power Engineering

# Strength of Materials Supplement for Power Engineering 

BCIT | School of Energy | Power Engineering

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## About the Book

Strength of Materials is a topic taught in all Mechanical Engineering related programs and as such is part of the Power and Process Engineering curriculum in British Columbia Institute of Technology (BCIT). The course continues developing the engineering foundation set in Principles of Statics and Applied Mechanics, taught to second year Power and Process students in Level 1. Successful completion of the course helps Power Engineering (PE) graduates fast track their careers as the topic is part of their $2^{\text {nd }}$ and $1^{\text {st }}$ class examinations curriculum.

The delivery of Strength of Materials course to Power Engineering students is centered around the open textbook written by Dr. Barry Dupen, Associate Professor in the Mechanical Engineering Technology Department of Indiana University - Purdue University Fort Wayne. The text book is licensed under Creative Commons Attribution ${ }^{1}$ and is available for download here:

Applied Strength of Materials for Engineering Technology
It was Dr. Dupen's intention to not include end of the chapter questions as he provides new sets with every course delivery. This gave us the opportunity and challenge to create our own problem sets. The problems were developed (or selected) in such way that they are relevant to Power Engineers. At the same time, when we felt that further summaries or procedures would enhance the learning process and help to our students, we added them. The focus of the "Supplement" is mostly on the "Applied" attribute of Strength of Materials discipline while still summarizing some theoretical aspects.

## Credits

- The cover picture, my own image, shows the "HÜTTE" Engineering Handbook and the slide rule used by my father in his civil engineering career.
- Selected images used in the Supplement were taken from Public Domain and credited appropriately.
- The rest of the images are my own work.


## Preface

A fact well know to students and acknowledged by educators is that college and university textbooks are expensive and not having access to one may affect students' success. Typically, these books establish the knowledge foundation of lucrative careers and are also part of the technical library of many working professionals.

In BCIT we have delivered this course following the "Applied Strength of Materials" textbook written by Robert L. Mott. In my years of teaching the subject, alongside with the students, we have used the $4^{\text {th }}$ and $5^{\text {th }}$ edition of the textbook. I often noticed that students could not afford the bookstore textbook or they had "international" paperback editions. When the cost of such a textbook was about $15 \%$ of the original one, I found it hard to reason with them. For a few years the textbook was discontinued by the publisher and our institute managed to secure internal publishing rights on a cost-recovery basis. Things changed again when the textbook was upgraded to the $6{ }^{\text {th }}$ Edition, undoubtedly improved but also more expensive. And this brings us to the current textbook...

I was pleasantly surprised to find a first-rate Strength of Materials textbook that covers all the topics that our students need (and then some)... and free. The textbook that we follow in this course is written by Dr. Barry Dupen, Associate Professor in the Mechanical Engineering Technology Department of Indiana University - Purdue University Fort Wayne.

## Acknowledgements

The adoption of this textbook and the further development of study materials for our students was made possible with support from the Open Education Working Group at BCIT. Funding for this project was provided through an OER grant from the B.C. Open Textbook Project.

I would like to take this opportunity to thank Dr. Barry Dupen for sharing his engineering knowledge and work with the rest of us and for making this book freely available to all students.

I personally feel I'm indebted to the Pearson Prentice Hall publishers and the author of Applied Strength of Materials textbook, Robert L. Mott, Professor Emeritus. While lecturing with this textbook, not only that I witnessed students improve their critical thinking and problem solving skills, but also helped me transfer my university acquired knowledge to tangible engineering solutions.

Special appreciation goes to my colleagues:

- Mr. Serhat Beyenir - for encouragement, support and help when I needed it
- Dr. Sanja Boskovic - for inspiration, motivation, and for reviewing this work

To all students who joined me in this journey, thank you!

## Editors

The following students brought suggestions and contributions to this book:
2017-2018 Power and Process, $2{ }^{\text {nd }}$ year students

| Kevin Abian | Erfan Atrchi | Bir Bhatia | Andriy Chervatyuk | Robert Dean |
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| Eric Miska | Akheem White | Mitchell Hudson | Aleksey Bykov | Neil Cahoon |
| Gundeep Gill | Michael Huang | Colton Jansen | Troy Moltz | Ryley Partington |

Main Body

## Introduction and Units

## Units

## Learning Objectives

At the end of this introductory chapter you should be able to

- Demonstrate familiarity with the study procedure, performance expectations and components of the course
- Perform units conversions problems within SI and US Customary systems


## Study procedure

Delivery of this course is based on the Applied Strength of Materials for Engineering Technology, by Dr. Barry Dupen. This resource will be referred to as the "textbook".

To complement the textbook students have access to the current resource, further identified as "supplement". This consists of summaries of main concepts developed in the textbook and assigned problems.

For best results students should adhere to the following sequence:

1. Before class, study the theory and review the sample problems in the textbook. Some topics were already covered in Applied Physics but you will benefit from a brief review.
2. To reinforce the concepts, review the key notes in the supplement. Take notes of the concepts you found challenging and ask for clarifications in class.
3. Classroom lectures:

- Instructor will review the theoretical concepts and answer questions
- Instructor will demonstrate solving selected problems. When needed, instructor's notes will be published on line.
- Students will solve assigned problems in small groups, with guidance from instructor

4. Individual work

- Students will solve assigned problems on their own, for self-evaluation.
- Instructor will provide guidance and feedback during posted office hours or Tutorial Sessions


## Course evaluation:

- Each chapter will be assessed through quizzes or assignments.
- Midterm and Final examinations. Combined passing score is $60 \%$.
- Attendance will be monitored but is not mandatory.


## Recommendations

Strength of Materials is a "methodical" discipline. This means that it deals in general with standard/ classical questions that usually have an established method of solving them. When solving problems students often follow steps and procedures that were previously demonstrated in class or in the textbook. These approaches are logical and never students would be expected to memorize them. However, it is important for students to practice solving questions on their own since this will help them see patterns in questions, provide them with problems solving experience and help them complete the exercise in the allotted time.

For best results, students are encouraged to work after classes between 2 and 3 hours for each hour of lecture. This effort will be different for each student. To manage your time more efficiently consider attending the weekly scheduled tutorials.

## Units and conversions

Like in many other engineering disciplines calculations may be performed in both systems of units, US Customary and SI. While Canada has officially adopted the SI (metric) system in 1970, economic cooperation with US companies requires engineering graduates to be fluent in both systems. Some computational software that you will use may be available only in US Customary units, being developed in US, and mostly for American users. It is therefore imperative to be able to complete calculations in both systems of units and to be able to convert between systems.

[^0]In the metric system prefixes are added to base and derived units to form names and symbols that are multiples of SI units. The following table shows the commonly used SI prefixes.

| Prefix | Symbol | Multiplying Factor |
| :--- | :--- | :--- |
| Giga X | GX | $10^{9}=1000000000$ |
| Mega X | MX | $10^{6}=1000000$ |
| Kilo X | kX | $10^{3}=1000$ |
| Hecto X | hX | $10^{2}=100$ |
| Deca X | daX | $10^{1}=10$ |
| Base SI Unit " $X$ " | "X" can be m, g, W, J, etc. | $10^{0}=1$ |
| Deci | dX | $10^{-1}=0.1$ |
| Centi | cX | $10^{-2}=0.01$ |
| Milli | mX | $10^{-3}=0.001$ |
| Micro | $\mu \mathrm{XX}$ | $10^{-6}=0.000001$ |

There are different ways to perform units conversions but in the end, they all lead to the same answer. The following are simple examples to demonstrate the procedure.

```
Examples - SI system:
```

1. Convert 0.2 km to cm
$0.2 \mathrm{~km} \times \frac{1000 \mathrm{~m}}{1 \mathrm{~km}} \times \frac{100 \mathrm{~cm}}{1 \mathrm{~m}}=20000 \mathrm{~cm}$

- When performing SI conversions it is easy to see if your answer is reasonable or not. For instance if you move from a large unit (kilo) to a smaller one (centi), the resulting value should be greater.
- Looking at Fig. 1, you may also consider moving the decimal point to the right, three steps from Kilo to base and two more steps from base to your final answer. This is an alternative approach to performing SI conversions.

2. Convert 50000 cW to kW

$$
50000 \epsilon W \times \frac{1 \mathrm{~W}}{100 \epsilon W} \times \frac{1 \mathrm{~kW}}{1000 \mathrm{~W}}=0.5 \mathrm{~kW}
$$

- Note that some units may be presented with a less commonly used prefixes. For instance, while "centimeter" is frequently used, "centiwatts" not so much. However, you should be
able to identify the prefix and the unit it applies to.

3. Convert $300000 \mathrm{~cm}^{3}$ to $\mathrm{dam}^{3}$

$$
300000 \mathrm{~cm}^{3} \times \frac{1 \mathrm{~m}^{3}}{(100 \mathrm{~cm})^{3}} \times \frac{1 \mathrm{dam}^{3}}{(10 \mathrm{~m})^{3}}=0.0003 \mathrm{dam}^{3}
$$

- You may look at this conversion as follows:


## $300000 \mathrm{~cm}^{3} \times \frac{1 \mathrm{~m} \times 1 \mathrm{~m} \times 1 \mathrm{~m}}{100 \mathrm{~cm} \times 100 \mathrm{~cm} \times 100 \mathrm{~cm}^{2}} \times \frac{1 \mathrm{dam} \times 1 \mathrm{dmm} \times 1 \mathrm{dam}}{10 \mathrm{~m} \times 10 \mathrm{~m} \times 10 \mathrm{~m}}=0,0003 \mathrm{dmm}$

- Pay extra attention when using powers, as in volume or area conversions.

For the purpose of this course most of the US Customary conversions will deal with linear dimensions. The conversion factors we use are presented in Appendix A. It is desirable to remember the most used factors such as $1 \mathrm{ft}=12$ in or $1 \mathrm{yd}=3 \mathrm{ft}$.

Examples - US Customary system
4. Convert 1.2 yards to inches

$$
1.2 y d \times \frac{3 f t}{1 y d} \times \frac{12 i n}{1 f t}=43.2 \mathrm{in}
$$

5. Convert 2 square feet to square inches

$$
2 f t^{2} \times \frac{12 \times 12 i n^{2}}{1 f t^{2}}=288 i n^{2}
$$

## Assigned Problems

Problem 1: The hoop stress in a pressure vessel is calculated with the formula

$$
\sigma_{\text {hoop }}=\frac{p \times d_{i}}{2 \times t}
$$ where $p$ is the design pressure, $d_{\mathrm{i}}$ is the inside diameter and $t$ is the wall thickness.

1. If $p=4450 \mathrm{kPa}, d_{\mathrm{i}}=1.8 \mathrm{~m}$ and $t=20 \mathrm{~mm}$, determine the hoop stress in the wall, in MPa.
2. If $p=645 \mathrm{psi}, d_{\mathrm{i}}=6$ feet and $t=3 / 4 \mathrm{in}$, determine the hoop stress in the wall, in ksi.

Problem 2: To determine the dead load on a foundation you are required to estimate the weigh of a spherical $\operatorname{tank}\left(\mathrm{V}=4 / 3 \pi \mathrm{r}^{3}\right)$, full with a liquid of given density. Tank mass is negligible compared to the mass of the product. Determine its weight based on the following:

1. Diameter $=200 \mathrm{~cm}$, density $=1.12 \mathrm{~g} / \mathrm{cm}^{3}$. Answer in N.
2. Diameter $=80 \mathrm{in}$., density $=70 \mathrm{lb} / \mathrm{ft}^{3}$. Answer in lb.

Problem 3: Suggest one improvement to this chapter.
The improvements have to be specific and clear, for example:

- correct this typo
- replace this phrase with this
- add this explanation to this section
- add this problem to the chapter problems
- etc

You may use screen captures to identify the section that you would like improved or expanded.

## Stress and Strain

## Stress-Strain

## Learning Objectives

After completing this chapter you should be able to:

- Define normal and shear stress and strain and discuss the relationship between design stress, yield stress and ultimate stress
- Design members under tension, compression and shear loads
- Determine members deformation under tension and compression


## Mechanical stress

This section discusses the effects of mechanical loads (forces) acting on members. Next chapter will cover the effects of thermal loads (thermal expansion).

> Normal, tensile and compressive stresses

Tension or compression in a member generate normal stresses; they are called "normal" because the cross-section that resists the load is perpendicular (normal) to the direction of the applied forces. Both tensile and compressive stresses are calculated with:

$$
\text { Normal Stress, } \sigma=\frac{\text { Normal Force }, P}{\text { Area }, A}
$$

If a member has a variable cross-section, the area that must be used in calculations is the minimum cross-sectional area; this will give you the maximum stress in the member, which ultimately will govern the design.

```
Shear stresses
```

In shear the cross-section area that resists the load is parallel with the direction of applied forces. In addition to that, when estimating the shear area you must factor in how many cross-sections contribute to the overall strength of the assembly.

For instance, if you consider the pin of a door hinge as subjected to a shear load, you have to count how many cross-sections resist the load.

The formula for calculating the shear stress is the same:

$$
\text { Shear Stress, } \tau=\frac{\text { Shear Force, } P}{\text { Total Shear Area, } A}
$$

In a punching operation the area that resists the shear is in the shape of a cylinder for a round hole (think of a cookie cutter). Therefore the area in shear will be found from multiplying the circumference of the shape by the thickness of the plate.


## Please note:

When looking at textbook figures you will observe that two forces are indicated. This does not mean that the force you use in the formula is ( $2 \times$ Force P ), but simply indicates that one is the Action force and the second one is the Reaction.

## Strain and modulus of elasticity

```
Normal strain
```

A member in tension or compression will elastically deform proportional with, among other parameters, the original length. Strain, also called unit deformation, is a non-dimensional parameter expressed as:

$$
\text { Strain, } \varepsilon=\frac{\text { Change in Length }}{\text { Original Length }}=\frac{\Delta L}{L}
$$

If you choose to use a negative value for compression strain (reduction in length) then you must also express the equivalent compression stress as a negative value.

Modulus of elasticity

The stress - strain curve is generated from the tensile test. Over the elastic region of the graph the deformation is direct proportional with the load. Dividing the load by the cross-section area (constant) and the deformation by the original length (constant) leads to a graphical representation of Strain vs. Stress. The constant ratio of stress and strain is Young's Modulus or Elastic Modulus, a property of each material.

$$
\text { Elastic Modulus, } E=\frac{\text { Stress, } \sigma}{\text { Strain, } \varepsilon}
$$

## Elastic deformation

Combining the above two relations for strain and Modulus of Elasticity leads to a unified formula for elastic deformation in tension or compression.

$$
\Delta L=\delta=\frac{\sigma \times L}{E}=\frac{F \times L}{A \times E}
$$

This relation is applicable to members with uniform cross-sections, homogeneous material, subject to tensile or compressive loads that results in stresses below the proportional limit (straight line in the $\sigma-\varepsilon$ curve).

## Design stress and safety factors

These topics were covered in $1^{\text {st }}$ year Strength of Materials and are presented here as a brief review.
Members subjected to an excessive stress may fail by breaking, when actual working stress is greater than the ultimate stress, or due to excessive deformation that renders then inoperable. Consider a heavy condensate line that sags beyond an acceptable limit and while it doesn't break, the flange connections at the end of the lines will develop leaks due to angular movement.

Design stress, $\sigma_{\mathrm{d}}$, is the maximum level of actual/working stress that is considered acceptable from a safety point of view. The design stress is determined by:

- Material properties, Ultimate Tensile Strength or Yield Strength, depending if breakage must be avoided or deformation must be limited
- Safety factor (or design factor) N, ratio of maximum strength to the intended load.

The safety factor is chosen by the designer based on experience, judgment AND guidelines/rules from relevant codes and standards, based on several criteria such as risk of injuries, design data accuracy, probability, industry standards, and last but not least, cost. Safety factors standards were set by structural engineers, based on rigorous estimates and backed by years of experience. Standards are continuously evolving reflecting new and improved design philosophies. Example:

- published by ANSI / AISC , such as Specification for Structural Steel Buildings


## Design cases

When solving problems students may encounter different scenarios. While the theoretical concepts are the same, the paths to final answers may be different, as required by each approach.

1. Estimating if a design/construction is safe or not
2. Given: loads magnitude and distribution, material properties, member shape and dimensions
3. Find: actual stress and compare to the design stress; alternatively find the safety factor and decide if it is acceptable based on applicable standards
4. Selecting a suitable material
5. Given: loads magnitude and distribution, member shape and dimensions
6. Find: what material type or grade will provide a strength (yield or ultimate) greater than required, while considering the selected or specified safety factor
7. Determining the shape and dimensions of member's cross-section
8. Given: loads magnitude and distribution, material properties
9. Find: the shape and dimensions of the member so that actual cross-sectional area is greater than minimum required.
10. Evaluating maximum allowable load on a component
11. Given: load type and distribution, material properties, member shape and dimensions
12. Find: maximum load magnitude that leads to an acceptable stress

## Members made from two different materials

There are cases when a member under normal stresses is made out of two (or more) materials. One of the objective of such problems is to find the stress in each component.

For example, you may have a short column made from a steel pipe filled with concrete, as in the figure. Given the total load, materials properties and geometrical dimensions, we must find the individual stress in each component.


Both, the steel pipe and the concrete core work together in supporting the load therefore we must find additional relations that combine the two problems into one. Typically, we look for:

- a relation that describes the force distribution between the two materials
- a relation that correlates the deformations of each material

For this particular problem we may say that:
Equation 1: Total load $\mathrm{P}=$ load supported by steel P steel + load supported by concrete P concrete
therefore $\quad \mathrm{P}=$ Stress $_{\text {steel }} \times$ Area steel + Stress $_{\text {concrete }} \times$ Area concrete
Equation 2: The deformations of both materials are the same
therefore Strain steel $=$ Strain concrete
Considering that Elastic Modulus = Stress / Strain, equation (2) yields a relation between the stress and elasticity of both materials

$$
\sigma_{\text {steel }}=\sigma_{\text {concrete }} \times \frac{E_{\text {steel }}}{E_{\text {concrete }}}
$$

Substituting this last relation into equation (1) and solving for Stress concrete leads to a relation as follows

$$
\sigma_{\text {concrete }}=\frac{F \times E_{\text {concrete }}}{A_{\text {steel }} \times E_{\text {steel }}+A_{\text {concrete }} \times E_{\text {concrete }}}
$$

Further, Stress steel can be found.
Note that depending on the problem, the original two relations may be different therefore a full step-by-step derivation may be required each time.

## Reasonable answers

When solving normal stress - strain problems, especially in the SI system, you should be able to judge if your answers are reasonable or not.

Example: A 1 m long, 20 mm diameter, A 36 Carbon Steel bar (Materials Properties in Appendix B, Table B2) suspends a 6 tons load. Evaluate the stress and the strain in the bar.

$$
\begin{aligned}
\sigma & =\frac{P}{A}=\frac{6000 \mathrm{~kg} \times 9.81 \mathrm{~m} / \mathrm{s}^{2}}{\frac{\pi}{4} \times 0.020^{2} \mathrm{~m}^{2}}=\frac{58.86 \mathrm{kN}}{0.0003 \mathrm{~m}^{2}}=187.5 \mathrm{MPa} \\
\varepsilon & =\frac{\sigma}{E}=\frac{187.5 \mathrm{MPa}}{207 \mathrm{GPa}}=0.0009
\end{aligned}
$$

Note that typically loads are in kN , cross-section areas in $10^{-3} \mathrm{~m}^{2}$ and resulting stresses in MPa.
Also, since Elastic Moduli are in GPa, the strain (non-dimensional) will be in range of $10^{-3}$. This bar will stretch 0.9 mm under the given load.

## Assigned Problems

When solving these questions you are required to use the textbook Appendices. They are valuable references for material properties, geometrical dimensions, etc.

Problem 1: A condensate line 152 mm nominal size made of schedule 40 carbon steel pipe is supported
by threaded rod hangers spaced at 2.5 m center-to-center. The hangers are carbon steel, 50 cm long, with a root diameter of 12 mm . Calculate the stress and the strain in the hangers. Use $\mathrm{E}=200 \mathrm{GPa}$ for the hangers material.

Problem 2: A clevis fastener with a $1 / 2$ inch pin is used in a shop lifting machine. If the pin is made of A36 steel determine the maximum safe load, using a safety factor of 2.5 based on the yield strength.

Problem 3: A boiler is supported on several short columns as indicated in the figure, made out of Class 35 gray cast iron. Each column supports a load of 50 tonnes. The required safety factor for this construction is 3 . Are the columns safe?


Use the following dimensions: $\mathrm{A}=30 \mathrm{~mm}, \mathrm{~B}=80 \mathrm{~mm}, \mathrm{C}=50 \mathrm{~mm}, \mathrm{D}=140 \mathrm{~mm}$
Problem 4: A tension member in a roof truss is subject to a load of 25 kips. The construction requires using $\underline{L 2 \times 2 \times 1 / 4}$ angle, with a cross-section of 0.944 in $^{2}$. For building-like structures American Institute of Steel Construction recommends using a design stress of $0.60 \times$ Sy. Using Appendix B table B2 specify a suitable steel material.

Problem 5: A tie rod hydraulic cylinder as in the figure is made from a 6 inch Schedule 40 stainless steel pipe, 15 inches long. The six tie rods are 1/2-13 UNC threaded rods with a root diameter of 0.4822 inch and a thread pitch of 13 TPI. When assembling the cylinder a clamping force equivalent to one full nut turn from hand-tight position is required.


Determine the stress in the cylinder and in the tie rods. Also calculate the strain in each component using an elastic modulus of $E_{s s}=28 \times 10^{6}$ psi and $E_{\text {rod }}=30 \times 10^{6}$ psi.

Problem 6: Suggest one improvement to this chapter.

## Thermal Expansion Stress

## Thermal Expansion

## Learning Objectives

At the end of this section you should be able to calculate problems involving

- Unrestricted thermal expansion
- Restricted thermal expansion

All materials subject to a temperature change will expand or contract proportional with their length and temperature difference. Some materials will expand or contract more than others; the qualitative property that indicates how much will they expand is known as the Linear Thermal Expansion Coefficient ( $\boldsymbol{\alpha}$ ), measured in $\mathrm{m} /\left(\mathrm{m}^{\circ} \mathrm{C}\right.$ ) or (in/in ${ }^{\circ} \mathrm{F}$ ). Also units like $1 /{ }^{\circ} \mathrm{C}$ or $1 /{ }^{\circ} \mathrm{F}$ can be used.

The change in length due to thermal expansion is calculated with:

$$
\Delta L(\text { or } \delta)=\alpha \times L \times \Delta T
$$

where $\delta$ is the change in length, L is the original length (makes sure both are in the same units) and $\Delta \mathrm{T}$ is the temperature difference.

For example if steel has a thermal expansion coefficient of $11.7 \times 10^{-6} 1 /{ }^{\circ} \mathrm{C}$ it means that a 1 m long bar subject to a temperature increase of $1^{\circ} \mathrm{C}$ will expand $11.7 \times 10^{-6} \mathrm{~m}$, or 0.0117 mm . This may seem like a negligible amount but if you consider a steam pipe of 50 m long installed at $12^{\circ} \mathrm{C}$ and operating at $212^{\circ} \mathrm{C}$ ( 2000 kPa saturation pressure), the thermal expansion would be equivalent to 11.7 cm , or an equivalent strain of 0.002 . This is very important for the piping designers because they have to allow for this expansion or factor it in the stress calculations.

Volumetric thermal expansion of solids (isotropic materials) is calculated in a similar way using ( $3 \times \alpha$ ) as expansion coefficient. When calculating liquids volumetric expansion, the volumetric expansion coefficient is $\beta$, with typical values as listed in The Engineering Toolbox.

## Pipelines expansion

Typically pipelines are relatively long and may see a significant temperature increase between installation and operating temperatures. As result, high magnitude thermal expansion stresses may develop if the supports are not adequately designed. In addition to that, the expansion of the pipe increases the load on machinery and vessels nozzles.

## Pipe cold springing

There is an abundance of articles and discussions on this topic in piping design groups, easily accessible through an internet search using key word strings "pipe cold springing" or "pipe cold pull"; it is also addressed in ASME B31.3.
"Pipe cold springing is defined as the process of intentional deformation (usually accomplished by cutting short or long the pipe runs between two anchors) of piping during assembly to produce a desired initial displacement and stress. It is also defined as the intentional stressing and elastic deformation of the piping system during the erection cycle to permit the system to attain more favorable reactions and stresses in the operating condition." [1]

Operating engineers are advised to be familiar with this practice since it may be used in steam pipes. There have been circumstances when hired contractors when disassembling steam lines complained about lines not being fitted properly; the pipes would sprung back when unbolted. Avoid costly repairs and unnecessary alterations by being familiar with this procedure and by knowing your plant.

## Thermal stresses in composite bars

"The composite tube consists of two different alloys metallurgically bonded together to achieve good thermal transfer properties. One alloy is used to withstand corrosion, while the other is often an approved pressure vessel material.

Typical applications for composite tubes are steam boilers with corrosive conditions, such as:

- Black liquor recovery boilers (BLRB)
- Syngas coolers
- Waste heat boilers
- Waste-to-energy boilers

Composite tube (compound tube) are suitable for applications where the conditions outside and inside the tube require material properties that cannot be met by one material only." [2]

While Power Engineering students may not see a direct application of these principles, the following types of problems are part of their $2^{\text {nd }}$ and $1^{\text {st }}$ class curriculum.

## Case A

The following diagram represent a typical restricted thermal expansion scenario, with compound bars:


Given all the materials properties and dimensions, the objective is to calculate the stress in each section when the temperature is increased by a given $\Delta \mathrm{T}$.

When the bars are heated, each will attempt tend to expand equivalent to their unrestricted $\Delta \mathrm{L}$. Given that the expansion is restricted, each bar will be subject to compression which in turn generates compression stresses. The sum of the two corresponding (yet imaginary) compression deformations will be equal to the sum easily quantifiable unrestricted thermal expansions. Furthermore, considering that the forces applied by each bar are the same (static/balanced system), this compression force can easily be calculated. Factoring in the cross-sectional areas of each bar leads to finding the stress developed in each material.

## Case B

In the second scenario, a bar is pinned at both ends inside a tube of a different material. When heated up, one material would expand freely more than the other. The lower expansion material will be pulled outwards in tension by the second one that attempts to expand more. In turn, the material that would freely expand more it is pulled in (compression) by the material that expands less. See the following figure for clarification.


Objective of this exercise is to find the stresses generated in each material. The approach at solving this problem is as follows.

- When heated up, the brass tube is pulled by the steel rod, generating a compression stress in
the tube. The brass tube restricted deformation will be ( $\mathbf{d} \mathbf{L}_{\mathbf{b}}-\boldsymbol{Y}$ ), where Y is the actual combined deformation of the composite bar.
- Similarly, the steel bar is pulled out by the brass tube, generating a tensile stress. The actual (restricted) deformation of the steel bar is $\left(\boldsymbol{Y}-\mathbf{d} \mathbf{L}_{\mathbf{s}}\right)$.
- Note than in the above dL is the free thermal expansion of each material.
- From the diagram, $\left(\mathbf{d} \mathbf{L}_{\mathbf{b}}-\mathbf{Y}\right)+\left(\boldsymbol{Y}-\boldsymbol{d} \mathbf{L}_{\mathbf{s}}\right)=\boldsymbol{d} \mathbf{L}_{\mathbf{b}}-\boldsymbol{d} \mathbf{L}_{\mathbf{s}}$
- Substitute in the above $d L=\alpha \times L \times \Delta T$ for each material, divide equation by $\mathrm{L}=$ initial length and find:
- $\varepsilon_{\text {brass }}+\varepsilon_{\text {steel }}=\left(\alpha\right.$ brass $\left.-\alpha_{\text {steel }}\right) \times \Delta T$
- Substitute $\mathrm{E}=\sigma / \varepsilon$ in the above for each material and the resulting equation represents a relation between the stress in each material, function only of the known elastic/thermal expansion properties and the temperature difference.

$$
\circ\left(\sigma_{\text {brass }} / E \text { brass }\right)+\left(\sigma_{\text {steel }} / E \text { steel }\right)=\left(\alpha_{\text {brass }}-\alpha_{\text {steel }}\right) \times \Delta T \quad(\text { eqn. B1 })
$$

- The second equation comes from the observation that outward pull force of the brass is equal to the inward pull of the steel. This can be expressed as:

$$
\begin{equation*}
\circ \sigma \text { brass } \times A \text { brass }=\sigma_{\text {steel }} \times A_{\text {steel }} \tag{eqn.B2}
\end{equation*}
$$

- Solve equation B2 for $\boldsymbol{\sigma}$ brass and substitute into equation B1. Solve equation B1 for $\boldsymbol{\alpha}$ steel and your final result is dependent only on materials properties, cross-sections and temperature difference.
- Once $\boldsymbol{\sigma}$ steel answer is found, go back to equation B2 and find $\boldsymbol{\sigma}$ brass.
- This may appear as a math/algebra demanding problem, and it is; however, it is a classical problem with a standard solution which means that every question will be solve following the same approach.


## Assigned Problems

When solving the following problems find the required data in the textbook appendices, provided external resources or other reputable sources; always quote the source.

Problem 1: A firetube boiler is powered using bunker fuel oil. The storage tank is of an open top construction, 2 m diameter and 3 m height. Oil is added when the ambient temperature is $10^{\circ} \mathrm{C}$. During start-up the temperature suddenly rises to $35^{\circ} \mathrm{C}$. How many centimeters below the tank top can you fill in the tank, so that you maximize the oil volume while avoiding any spillage? Coefficient of linear expansion of tank material is $12 \times 10^{-6} /{ }^{\circ} \mathrm{C}$ and volumetric expansion coefficient for oil is $9 \times 10^{-4} /{ }^{\circ} \mathrm{C}$.

Problem 2: During installation, a turbine casing bolt is heated to $250^{\circ} \mathrm{C}$ and the nut is tightened so that no stress is produced (hand-tight). When it cools down to the operation temperature of $50^{\circ} \mathrm{C}$ the bolt adequately secures the assembly. Determine the tensile stress and strain in the bolt and the force carried by the bolt. The bolt effective length is 300 mm , diameter 50 mm and $\mathrm{E}_{\text {bolt }}=200 \mathrm{GPa}$.

Problem 3: A new above ground pipeline will transport crude oil from Northern Alberta, south. To compensate for thermal expansion each straight section of the pipeline will be equipped with corrugated
expansion joints that allow 23 mm axial expansion and 18 mm axial compression (figure). The pipeline will be installed early summer when ambient temperatures may conservatively assume to be $23^{\circ} \mathrm{C}$. The pipe is DN 600 Sch 40 and Carbon Steel material.

Determine the maximum straight pipe length between two anchor points (in m) for extreme Alberta temperatures while assuming that due to bush-fires the pipe metal temperature can reach as high as $100^{\circ} \mathrm{C}$. For your specified pipe length what would be the maximum stress developed in the material if the thermal expansion is restricted?


Problem 4: An 8" Schedule 40 straight length of steam pipe is fitted between two fixed anchor supports with no allowance for expansion. If the compressive stress in the pipe must be limited to 50.7 ksi when in operation at $430^{\circ} \mathrm{F}$, determine the initial tensile stress that must be applied during installation at $60^{\circ} \mathrm{F}$. What equivalent tensile force is required by this cold-springing installation?

Problem 5: A single-pass double pipe heat exchanger is constructed using $1^{\prime \prime}$ nominal thickness ASTM B88 Type K Copper tubing for the internal tube and 2 " nominal size, medium wall thickness,
steel tubing for the external shell. The length of the heat exchanger is 24 ". The heat exchanger is assembled stress free at $20^{\circ} \mathrm{C}$ but in operation the tubes wall temperatures reach $120^{\circ} \mathrm{C}$. Determine the stresses generated by thermal expansion in both, steel and copper tubing. Use:

- $\alpha_{\text {steel }}=6.5 \times 10^{-6} \mathrm{in} / \mathrm{in}^{\circ} \mathrm{F} ; \mathrm{E}_{\text {steel }}=30 \times 10^{6} \mathrm{psi}$; dimensional data from $\underline{\text { The Engineering }}$ Toolbox Steel Tubes
- $\alpha_{\text {copper }}=9.4 \times 10^{-6} \mathrm{in} / \mathrm{in}^{\circ} \mathrm{F}$; $\mathrm{E}_{\text {copper }}=17 \times 10^{6} \mathrm{psi}$; dimensional data from Appendix D5 or The Engineering Toolbox Copper Tubes

Problem 6: Recommend one improvement to this chapter.

## Pressure Vessels

## Vessels

## Learning Objectives

At the end of this chapter you should be able to

- Identify thin wall or thick wall pressure vessels
- Discuss the difference between longitudinal and circumferential stress
- Demonstrate the derivation of the stress formulas in a thin wall pressure vessel
- Perform thin wall pressure vessel design calculations


## Thin-walled and thick-walled pressure vessels

The distinction between thin vs. thick wall pressure vessels is determined by the ratio between the mean radius of the vessel and the thickness of the wall. If this ratio is greater than 10 , the vessel is considered a thin wall pressure vessel. If the ratio is less than 10 , the vessel is considered a thick wall pressure vessel.

## $\frac{R_{i}\left(\text { Radius }_{\text {inside }}\right)}{t(\text { wall thickness })} \geq 10$ means Thin Wall P.V.

In operation, in a thin wall pressure vessel, stresses developed in the (thin) wall can conservatively be assumed to be uniform. These are the stresses students are familiar calculating using ASME Section I PG-27 or Section VIII Div. I UG-27. In fact, most of the pressure vessels power engineers will work with are of a thin-wall type.

In contrast, a thick wall pressure vessel develops a greater (circumferential) stress on the inside surface of the vessel and it reduces towards the outside diameter. The design calculations for this type of vessels are only covered in the ASME Section VIII (Pressure Vessels) code, Mandatory Appendix 1 (Supplementary Design Formulas).

## Development of stress formula in a pressure vessel

## Circumferential stresses (longitudinal joints)

The circumferential stress (or hoop stress) acting on a longitudinal cross-section is derived in the textbook as:

$$
\sigma_{\text {hoop }}=\frac{p \times d_{i}}{2 \times t}=\frac{p \times r_{i}}{t}
$$

Design problems most typically deal with finding the minimum required wall thickness, therefore the above formula is more useful expressed as:

$$
t=\frac{p \times d_{i}}{2 \times \sigma_{\text {hoop }}}=\frac{p \times r_{i}}{\sigma_{\text {hoop }}}
$$

Hoop stress formula from ASME Section VIII Div. 1 UG-27 is:

$$
t=\frac{P \times R}{S \times E-0.6 P}
$$

Efficiency "E" is a factor that accounts for loss of material strength due to welds or ligaments. Also note that applying "-0.6P" to the denominator leads to a thicker shell compared to the theoretical formula, and therefore more conservative (or safer). Before using the formula check if the relation is applicable (thin wall).

ASME Section I (Power Boilers) calculates the shell thickness only based on circumferential stress, as follows:

$$
t=\frac{P \times R}{S \times E-(1-y) P}+C
$$

The formulas are quite similar; in the above " y " is a temperature coefficient and C is an added allowance for corrosion or structural stability. Again, the code formula leads to a thicker shell than simply based on derivations.

Longitudinal stress (circumferential joints)

Longitudinal stress demonstrated and derived in the textbook is derived as:

$$
\sigma_{l o n g}=\frac{p \times r_{i}}{2 \times t} \text { or } t=\frac{p \times r_{i}}{2 \times \sigma_{\text {long }}}
$$

Note that longitudinal stresses are $50 \%$ of the hoop stresses and therefore they rarely govern the design. This is the reason ASME Section I does not even require evaluating this stress.

ASME Section VIII Div. 1 requires estimating the vessel thickness based on both stresses, and choosing the largest of the two values. Formula is:

$$
t=\frac{P \times R}{2 S \times E+0.4 P}
$$

Spherical pressure vessels

Spherical pressure vessel stress is calculated in the same way as the longitudinal stress. You may conclude that a spherical pressure vessel will require a thinner shell, theoretically one half, than a cylindrical pressure vessel operating at the same pressure and temperature, and therefore it would be a preferred shape. Reality is that while most of that is true, it is difficult to manufacture a spherical shell.

Follow the links for examples of pressure vessels:

- A pressure vessel constructed of a horizontal steel cylinder.
- Spherical gas container.
- LNG carrier ship.


## Assigned Problems

- The Pressure Vessel problems must be solved using the theoretical formulas developed in the textbook and NOT the ASME code formulas.
- Always check first if you are dealing with a thin-walled or a thick-walled pressure vessel.
- Thick wall formulas will be provided if necessary.
- Cylindrical vessels require both calculations (longitudinal and circumferential joints); you specify the final answer (minimum wall thickens or MAWP maximum allowable working pressure)

Problem 1: Derive in detail the formulas for longitudinal and circumferential stresses acting on a cylindrical pressure vessel. Briefly discuss the results.

Problem 2: A seamless pipe of 508 mm outside diameter is used as a header in a large power plant carrying steam at 2 MPa pressure. The standard lengths of pipe are butt-welded together to build a continuous pipe. The pipe material, SA-106 Grade C, has minimum Tensile Strength of 485 MPa and a
safety factor of 3.5 based is specified. The allowable stress for the butt-welds is 110 MPa . Specify the minimum pipe wall thickness.

Problem 3: A cylindrical tank $36^{\prime \prime}$ diameter and 12 feet long, is used as a compressed air accumulator. The tank is made of ASTM SA-36 rolled steel plate with a wall thickness of $3 / 4^{\prime \prime}$. Find the maximum allowable working pressure in the tank using a safety factor of 3.5 based on the Ultimate Strength.

Problem 4: A large spherical storage tank for compressed nitrogen is 8.6 m diameter and is constructed using AISI 1040 cold rolled steel plate of 12 mm thickness. The maximum pressure in the tank is 0.66 MPa . If a design factor of 4 based on the Yield Strength is required, does the tank meet the specifications?

Problem 5: Recommend one improvement to this chapter.

## Properties of Areas

## Learning Objectives

Upon completion of this chapter you should be able to

- Determine the centroid location of a given cross-section
- Calculate the moment of inertia for a given cross-section, with both SI and US Customary units

Finding the location of the centroid is needed when calculating the moment of inertia (or second moment of areas) of beams subjected to bending. For convenience, you may used the table provided in Appendix 1.

The geometric properties of areas for common shapes are given in textbook Appendix C. Common industrial shapes like W-beams and pipes are listed in Appendix D.

## Centroids of composite areas

Determining the location of the centroid of a composite area uses the concept of moment of an area; this is why textbooks may refer to this as "first moments of areas". Mathematically this principle is expressed as:

$$
Y=\frac{\sum\left(A_{i} \times y_{i}\right)}{\sum A_{i}}
$$

where:

- Y is the distance to the centroid from some reference axis. Commonly, the reference axis is the base of the figure.
- $A_{i}$ is the area of one part of the composite area. Typically, the composite areas are split into common shapes of known geometric properties, summarized in the textbook Appendix C.
- $\mathrm{y}_{\mathrm{i}}$ is the distance from the reference axis (commonly the base of the figure) to the centroid of each part of the composite figure.
- $\Sigma\left(\mathrm{A}_{\mathrm{i}}\right)$ is the entire area of the composite area.

When determining the location of a centroid please observe the following rules:

- If the cross-section has one axis of symmetry then the centroid will be located on this axis.
- if the cross-section has two axes of symmetry then the centroid will be located at the intersection of the two axes.
- If the cross-section is not symmetric about any axis then two calculations are required:
- one for determining the centroid location Y
- one for determining the centroid location X, commonly measured from the extreme left end. For this second calculation imagine that you rotate the figure $90^{\circ}$ counterclockwise and repeat the first calculation
- If the composite area has a part that is removed from the figure (a void), this missing part can be treated as a negative area.


## Moments of inertia of composite areas

In rotational kinetics we learned that the "rotational" moment of inertia of a flywheel (function of its mass, size and shape) represents a resistance to change in its motion. This moment of inertia multiplied by the angular acceleration $\alpha$, gives an inertia-moment reaction that attempts to balance the accelerating moment action (accelerating torque). In general, a moment of inertia is a resistance to change.

Beams are subject to bending and as a result they tend to deform (deflect). The moment of inertia of a beam cross-section can be related to the stiffness of the beam. The deflection of the beam is inverse proportional to the moment of inertia.

Formulas for moments of inertia of simple shapes are given in Textbook Appendix C. They will also be provided in the exam.

When dealing with a composite area, divide the shape into basic parts for which the moment of inertia can be easily calculated. The combined moment of inertia of the entire shape is the sum of moments of inertia of constituent parts plus their corresponding transfer term. The transfer term is calculated as the area of the part multiplied by the squared distance between the centroid of the part and the common centroid of the entire area. This transfer term represents the additional stiffness of each part due to its relative distance from the common centroid. The table given in Supplement Appendix 1 can be used for calculations; it is useful when the shape is more complex.

## Assigned Problems

When completing these exercises please make sure that you clearly identify and number the parts of your composite area.

Problem 1: Determine the moment of inertia about the vertical and horizontal centroidal axes for the following figure.


Problem 2: For the following cross-section determine the location (elevation) of the centroid and the moments of inertia with respect to the horizontal and vertical centroidal axes.


Problem 3: For the following figure determine Y, the vertical location of the centroid, and calculate the moment of inertia with respect to the horizontal centroidal axis..


Problem 4: Suggest one improvement to this chapter; this may include an original cross-section.

## Beam Reactions and Diagrams

## Diagrams

## Learning Objectives

At the end of this chapter you should be able to:

- Determine the reactions of simply supported, overhanging and cantilever beams
- Calculate and draw the shearing force and bending moment diagrams of beams subject to concentrated loads, uniform distributed loads and combinations of the two.


## Beams review

Beams are structural elements with various engineering applications like roofs, bridges, mechanical assemblies, etc. In general, a beam is slender, straight, rigid, built from isotropic materials, and most important, subjected to loads perpendicular to their longitudinal axis. If instead of perpendicular loads the same structural member would be subjected to longitudinal loads it would be called column or post. If the same member would be subjected to a torque, it would be called and treated as a shaft. Therefore, when identifying mechanical or structural components, consideration of the manner of loading is very important.

Note that when it comes to orientation, beams can be horizontal, vertical or any inclination in between (like submerged plates analyzed in fluid mechanics)... provided the loading is perpendicular to their major axis.

## Beam supports:

Symbol


- simple
- movable
- sliding



## Reactions



- Vertical reaction only
- Allows horizontal movement
- Allows rotation

- Vertical reaction
- Horizontal reaction
- Allows rotation

- Vertical reaction
- Horizontal reaction
- Moment reaction


## Beam Loads ${ }^{1}$ :

Point, also called

- concentrated


Uniform Distributed



Concentrated Moments


Beam types:

## Types <br> Diagram

Simple beams, or simply supported



Overhanging beams


Cantilever
beams


Compound beams


Continuous
beams


## Solving for beam reactions

When solving for reactions, the following steps are recommended:

1. Draw the beam free body diagram
2. Replace the uniform distributed load (if any) with the equivalent point load
3. Solve $\Sigma \mathrm{M}_{\mathrm{A}}=0$ (sum of moments about support A). This will give you $\mathrm{R}_{\mathrm{B}}$ (reaction at support B).
4. Solve $\Sigma \mathrm{M}_{\mathrm{B}}=0$. This will give you $\mathrm{R}_{\mathrm{A}}$.
5. Using $\mathrm{R}_{\mathrm{A}}$ and $\mathrm{R}_{\mathrm{B}}$ found at steps 3 and 4 check if $\Sigma \mathrm{V}=0$ (sum of all vertical forces) is satisfied.
6. Note that steps 4 and 5 can be reversed.
7. For a cantilever beam use $\Sigma \mathrm{V}=0$ to find the vertical reaction at the wall and $\Sigma \mathrm{M}_{\text {wall }}=0$ to find the moment reaction at the wall. There is no other equation to validate your results.

## Shear forces and bending moments diagrams

## Please note:

"Shearing forces are internal forces developed in the material of a beam to balance externally applied forces in order to secure equilibrium of all parts of the beam.

Bending moments are internal moments developed in the material of a beam to balance the tendency for external forces to cause rotation of any part of the beam." [3]

The shear force at any section of a beam may be found by summing all the vertical forces to the left or to the right of the section under consideration.

Similarly, the bending moment at any section of a beam may be found by adding the moments from the left or from the right of the section considered. The moment's pivot point is the location under consideration.

By convention, internal shearing forces acting downward are considered positive. They counteract upward external forces. Therefore, when representing the shear forces you can draw them in the direction of external forces. This is visually easier than following the sign convention.

Clockwise moments, conventionally, are considered negative while counter-clockwise moments are considered positive. When representing the bending moment variation, consult the following table showing qualitative bending moment curves dependent on the shape of the shear force graphs.

| Shear <br> Force <br> Type |  | Constant <br> positive <br> slope | Constant <br> zero <br> slope | Constant <br> negative <br> slope |
| :--- | :--- | :--- | :--- | :--- |
| S.F. <br> graph | + |  |  |  |
|  | - |  |  |  |
|  |  |  |  |  |
|  |  |  |  |  |


| Shear <br> Force <br> Type |  | Positive <br> increasing <br> slope | Positive <br> decreasing <br> slope | Negative <br> increasing <br> slope | Negative <br> decreasing <br> slope |
| :--- | :--- | :--- | :--- | :--- | :--- |
| S.F. <br> graph | + |  |  |  |  |
|  | - |  |  |  |  |
|  |  |  |  |  |  |
|  |  |  |  |  |  |
|  |  |  |  |  |  |

When drawing the shear force and bending moment diagrams, while the sign convention is important, consistency is crucial. For instance, consider a simple beam loaded with a point load applied on a UD load. Starting the diagrams at support A, looking towards the page, will generate the following:


Now, flip the beam horizontally $180^{\circ}$ (or change the observation point, looking at the beam from the opposite side) and draw the diagrams, starting from the same point $A$. The diagrams will appear as follows:


Note that, while the shear force diagrams appeared to be mirrored images (flipped horizontally), the bending moment diagram is not affected. Additionally, the most important result of this analysis, illustrates that maximum shear force and bending moment magnitudes will always be the same.

## Beam diagrams check points

When drawing the beam diagrams please observe the following:

## Shear Forces Diagrams:

- At the ends of a simply supported beam the shear force is zero.
- At the wall of a cantilever beam the shear force equals the vertical reaction at the wall. At the beam's free end the shear force is zero.
- On any beam segment where no loads are applied, the shear force remains constant
(horizontal line).
- A point load or reaction on a shear force diagram generates an abrupt change in the graph, in the direction of the applied load.
- A uniform distributed load acting on a beam is represented by a straight line shear force with a negative or positive slope, equal to the load per unit length.


## Bending Moments Diagram:

- At the ends of a simply supported beam the bending moments are zero.
- At the wall of a cantilever beam, the bending moment equals the moment reaction. At the free end, the bending moment is zero.
- At the location where the shear force crosses the zero axis the corresponding bending moment has a maximum value.
- The shape of the bending moment curve between two points on the beam is as shown in the above two tables.
- The change in bending moment between two points on the beam equals the area under the shear force diagram between the same two points.

The above guidelines will assist you in generating the beam diagrams; they also serve as a check.

## Assigned Problems

Calculate the beam reactions and draw the shear force and bending moment diagrams for the following beams.

When solving beam diagrams in class and at home you may check your answers by using this free online beam calculator: SkyCiv Cloud Engineering Software

Problem 1: State the maximum shear force and bending moment values.


Problem 2: State the maximum shear force and bending moment values.


Problem 3: A 24 meters long beam is simply supported at 3 meters from each end. The beam carries a point load of 18 kN at the left end and 22 kN at the right end of the beam. The beam weighs $400 \mathrm{~kg} /$ m . Sketch the beam diagrams and determine the location on the beam where the bending moment is zero.

Problem 4: A simple overhanging beam 112 ft long overhangs the left support by 14 ft . The beam carries a concentrated load of 90 kips 12 ft from the right end and a uniform distributed load of 12 kips/ ft over a 40 ft section from the left end. Sketch the beam diagrams and determine the shear force and the bending moment at a section 50 ft from the left end.

Problem 5: Suggest an improvement to this chapter.

## Beam Stress due to Bending Moments

## Bending Stress



After completing this chapter you should be able to:

- Use the flexure formula to calculate maximum bending stress
- Design beams carrying loads safely
- Determine the required Section Modulus of a beam
- Select standard structural shapes to be used in a given beam problem

Consider a simply supported beam subjected to external downward loads. The beam will deform (deflect) in such a way that the top surface of the beam cross-section will be under compression while the bottom surface will be in tension. At some location along the vertical axis of the beam, the stress will be zero; this location is the centroid of the cross-section, also called the neutral axis.


To determine the maximum stress due to bending the flexure formula is used:
$\sigma_{\max }=\frac{M \times c}{I_{x}}=\frac{M}{Z_{x}}$
where:

- $\sigma_{\max }$ is the maximum stress at the farthest surface from the neutral axis (it can be top or bottom)
- M is the bending moment along the length of the beam where the stress is calculated
- if the maximum bending stress is required then $M$ is the maximum bending moment acting on the beam
- $\mathrm{I}_{\mathrm{X}}$ is the moment of inertia about x (horizontal) centroidal axis
- c is the maximum distance from the centroidal axis to the extreme fiber (again, this can be to the top or bottom of the shape)
- $\mathrm{Z}_{\mathrm{x}}$ is called section modulus and is a term that combines the moment of inertia and the distance to the extreme fiber ( $\mathbf{Z}_{\mathbf{X}}=\mathbf{I}_{\mathbf{X}} / \mathbf{c}$ )

The flexure formula is valid if the following criteria are met:

- the beam is straight, relatively long and narrow and of uniform cross-section
- all the loads act perpendicular to the longitudinal axis of the beam
- the resulting stress is below the limit of proportionality of the material
- the beam material is homogeneous and has equal strength in tension and compression
- if the material has different strengths in tension and compression (example cast iron or other anisotropic materials) then separate calculations are required for both tension and compression surfaces
- no twisting, buckling or crippling occurs


## Design cases

Design problems may follow different scenarios:

- calculate the beam cross-sectional dimensions (find the minimum section modulus Z and choose a standard shape of greater stiffness), given the beam geometry, loading and material.
- select the beam material (find maximum working stress and choose a material of greater strength), given the beam dimensions, loading and dimensions/shape.
- determine if a beam is safe (find actual working stress and compare to design stress), given the beam dimensions, loading and material.


## Assigned Problems

Note: if not specified, use $\sigma_{\text {design }}=0.6 \times \sigma$ ys, where $\sigma_{\mathrm{Ys}}$ is the Yield Strength, from textbook Appendix B.

Problem 1: A simply supported beam, 9.9 meters long, is loaded with concentrated loads as follows:

- 40 kNa @ 1.2 m from left end
- $10 \mathrm{kN} @ 3.7 \mathrm{~m}$ from left end
- $10 \mathrm{kN} @ 6.2 \mathrm{~m}$ from left end
- $10 \mathrm{kN} @ 8.7$ m from left end

The beam is constructed using W200×100 I-beam profile from AISI-1020 cold rolled material. $\underline{\text { AISC }}$ recommends that the maximum bending stress for building-like structures under static loads be kept below $0.66 \times \mathrm{S}_{\mathrm{y}}$. Does this construction meet the design requirements?

Problem 2: A pipeline is simply supported above ground on horizontal beams, 4.5 m long. Each beam carries the weight of 20 m Sch 40 DN-600 pipe (see PanGlobal Academic Extract), filled with oil of 0.9 SG. Assuming that the load acts at the center of the beam, calculate the required section modulus of the beam to limit the bending stress to 140 MPa ; then select the lightest SI W-beam that satisfies the criteria.

Problem 3: The figure shows the cross-section of a beam built from aluminum 6061-T6. The beam is used as a 45 in. long cantilever. Compute the the maximum allowable uniformly distributed load it could carry while limiting the stress due to bending to one-fifth of the ultimate strength.


Problem 4: Design a walkway to span a newly installed pipeline in your plant. Rigid supports are available on each side of the pipeline, 14 ft apart. The walkway has to be 3.5 ft wide and be able to support a uniformly distributed load of $60 \mathrm{lb} / \mathrm{ft}^{2}$ over its entire surface. Design only the deck boards and the side beams. Use any timber sizes and material grades from textbook Appendix E or others of your own design.

Problem 5: Suggest one beam design problem that you would consider relevant and useful for Power Engineers.

## Beam Deflection

## Deflection

## Learning Objectives

Upon completion of this chapter you should be able to calculate:

- The radius of curvature of a deflected beam using theoretical relations
- The maximum deflection of a simply supported beam
- The maximum deflection of various beams using Formula Method and textbook Appendices

Elastic properties of materials are quantified through their Modulus of Elasticity. All materials are elastic to some extent, for example $\mathrm{E}_{\text {steel }} \approx 210 \mathrm{GPa}, \mathrm{E}_{\text {cast iron }} \approx 160 \mathrm{GPa}$, $\mathrm{E}_{\text {aluminum }} \approx 70 \mathrm{GPa}$, $\mathrm{E}_{\text {concrete }} \approx$ 40 GPa . In real situations beams subjected to external loads will deflect proportionally to the bending moment and inversely to their stiffness. The overall stiffness of a beam can be expressed as $\boldsymbol{E} \times \boldsymbol{I}_{\mathbf{c}}$ where $E$ can be regarded as the material stiffness and $I_{\mathrm{C}}$ as the cross-sectional, or geometrical stiffness.

## Radius of curvature

Review the derivation of the beam deflection covered in detail in Textbook Chapter 10. In practical situations, beam deformation is very small when compared to its length, and as a result the radius of curvature is relatively large.


This radius of curvature can be calculated with

where:

- E is the modulus of elasticity (resistance due to material properties)
- $I_{C}$ is the moment of inertia about the centroidal axis (resistance due to section geometry)
- M is the bending moment at the section of interest

If the beam is loaded in such a way that the bending moment is constant over a section of the beam (horizontal line in the BM diagram) then the deflection is a circular arc and the radius of curvature is constant.

Take a moment and analyze the above formula... increasing the beam stiffness ( $\mathrm{E} \times \mathrm{I}_{\mathrm{C}}$ ) will reduce the deflection (large R), while a greater bending moment leads to a smaller radius of curvature (greater deflection/sagging).

## Beam deflection

Consider a simply supported beam as in the above diagram. Once the radius of curvature is found, the maximum deflection (at mid span) can easily be geometrically calculated as follows:


## Formula method for simple cases

The Radius of Curvature formula is valid solely for cases where the bending moment is constant. For other cases, geometrical or integration based techniques are involved in determining the beam deflection. Results of these calculations presented in algebraic form are given in engineering handbook of formulas. Most common cases are summarized in textbook Appendix F.

When using "off-the-shelf" formulas, you must first match the beam geometry and loading to one of the given cases. If you are dealing with a more complex loading, such as point loads over-imposed on a distributed load, you can analyze the two loads separately and for the total deflection simply add the constituents.

## Assigned Problems

For each problem determine the maximum deflection using the beam equations and compare with the value found using the radius of curvature.

Loading \& Dimensions

$$
\text { - } \mathrm{P}=50 \mathrm{kN}
$$

- $\mathrm{a}=2 \mathrm{~m} ; \mathrm{b}=$ 3.5 m
- $P=5000 \mathrm{lb}$.
- $\mathrm{a}=2 \mathrm{ft}$.
- $\mathrm{L}=10 \mathrm{ft}$.
- $\mathrm{w}=250 \mathrm{lbs} /$ ft
- $\mathrm{L}=35 \mathrm{ft}$.
$\mathrm{w}=4400 \mathrm{~N} /$ m
- $\mathrm{a}=4 \mathrm{~m} ; \mathrm{b}=$ 8 m

- SS 304, cold rolled
- W $12 \times 30$
- Aluminum 6061-T6

Problem 4


Problem 5: Recommend one improvement to this chapter.

## Torsion in Round Shafts

## Torsion

## Learning Objectives

At the end of this chapter you should be able to complete torsion calculations using:

- General torsion equation
- Polar moment of inertia
- Modulus of elasticity in shear

Shafts are mechanical components, usually of circular cross-section, used to transmit power/torque through their rotational motion. In operation they are subjected to:

- torsional shear stresses within the cross-section of the shaft, with a maximum at the outer surface of the shaft
- bending stresses (for example a transmission gear shaft supported in bearings)
- vibrations due to critical speeds

This chapter will focus exclusively on evaluating shear stresses in a shaft.

## General torsion equation

All torsion problems that you are expected to answer can be solved using the following formula:

$$
\frac{T}{J}=\frac{\tau}{r}=\frac{G \times \theta}{L}
$$

where:

- $\mathrm{T}=$ torque or twisting moment, $[\mathrm{N} \times \mathrm{m}, \mathrm{lb} \times \mathrm{in}]$
- $J=$ polar moment of inertia or polar second moment of area about shaft axis, $\left[\mathrm{m}^{4}, \mathrm{in}^{4}\right]$
- $\tau=$ shear stress at outer fibre, [Pa, psi]
- $r=$ radius of the shaft, [m, in]
- $\mathrm{G}=$ modulus of rigidity (PanGlobal and Reed's) or shear modulus (everybody else), [Pa, psi]
- $\theta=$ angle of twist, [rad]
- $L=$ length of the shaft, [m, in]

The nomenclature above follows the same convention as PanGlobal Power Engineering Training System.

Most common torsion problems will indicate the transmitted power ( $\mathrm{kW} \mathrm{)} \mathrm{at} \mathrm{a} \mathrm{certain} \mathrm{rotational} \mathrm{speed}$ ( $\mathrm{rad} / \mathrm{s}$ or RPM). The equivalent torque can be found with:

$$
T[\mathrm{Nm}]=\frac{P[\mathrm{~W}]}{n[\mathrm{rad} / \mathrm{s}]}
$$

where $n[\mathrm{rad} / \mathrm{s}]=N[\mathrm{rev} / \mathrm{min}] \times 2 \pi / 60$.

## Polar moment of inertia

Similar to the moments of inertia that you learned before in rotational kinetics and bending of beams, the polar moment of inertia represents a resistance to twisting deformation in the shaft. General formulas for polar moment of inertia are given in Textbook Appendix C.

Note the difference between bending moments of inertia $I_{\mathrm{C}}$ and polar moments of inertia $J$, and use them appropriately. For instance, if you are dealing with a circular bar:

- $I_{\mathrm{C}}=\pi d^{4} / 64$, if the bar is used as a beam
- $J=\pi d^{4} / 32$, if the bar is used as a shaft


## Shear modulus

Called Modulus of Rigidity in PanGlobal and Reed's, the shear modulus is defined (similarly as E) as ratio of shear stress to the shear strain. It is expressed in GPa or psi and typical values are given in Textbook Appendix B. Typical values are lower than Young's Modulus E, for instance ASTM A36 steel has $E_{\text {A36 }}=207 G P a$ and $G_{\text {A36 }}=83 G P a$.

## Angle of twist

The torque deformation of a shaft due is measured by the twist angle at the end of the shaft. This angle of twist depends on the length of the shaft, as shown in the following figure:

by Barry Dupen
The angle of twist, [radians] is used in the general torsion equation and in estimating the shear strain, $\gamma$ (gamma), non-dimensional.


## Assigned Problems

Solve the following problems using the General Torsion Equation.

Problem 1: To improve an engine transmission, a solid shaft will be replaced with a hollow shaft of better quality steel resulting in an increase in the allowable stress of $24 \%$. In order to keep the existing bearings, the new shaft will have the same outside diameter as the existing, solid shaft. Determine:
(a) the bore diameter of the hollow shaft in terms of outside diameter
(b) the weight savings in percentage, assuming that the steel densities of both shafts are identical

Problem 2: A turbine - generator transmission is rated for 3500 kW at 160 RPM. The shafts, 180 mm diameter and 2 m long, are connected through a flanged coupling with 6 coupling bolts of 40 mm diameter arranged on a pitch circle of 340 mm . If the shaft shear modulus is 85 GPa determine:
(a) the maximum shear stress in the shaft
(b) the shear stress in the bolts

Problem 3: Two identical hollow shafts are connected by a flanged coupling. The outside diameter of the shafts is 240 mm and the coupling has 6 bolts of 36 mm each on a bolt circle of 480 mm . Determine the inside diameter of the hollow shafts, which results in the same shear stress in both, shafts and bolts.

Problem 4: A brass liner, 24 mm thick, is shrunk over a solid shaft of 220 mm diameter. Taking $G_{\text {steel }}=85 \mathrm{GPa}$ and Gbrass $=37 \mathrm{GPa}$, determine the maximum shear stress in the shaft and liner if the transmitted torque is $240 \mathrm{kN} \times \mathrm{m}$. Also determine the angle of twist if the shaft length is 3.4 m .

Problem 5: Suggest one improvement to this chapter.

## Bolted and Welded Joints

Joints

## Learning Objectives

At the end of this section you will be able to

- calculate the allowable load of bolted lap joints
- calculate the allowable load of welded joints


## Bolted joints

Two end plates bolted together and subject to symmetrical tensile loads react to the applied forces through the shear resistance of the bolts and the friction force developed between the plates. The friction force is difficult to evaluate since it depends on the relative roughness of the contact surface and may also be affected by environmental changes (bolts thermal expansion reduces the friction force). As a result, conservative load calculations rely only on the shear resistance of the bolts (or rivets); the extra joint capacity due to friction increases the safety factor.

There are various scenarios that may lead to bolted joints failure, all described in the textbook. When completing these calculations please note the following:

- nomenclature is listed in the textbook on page 6.
- material properties are taken from Textbook Appendices B3 and B4

The following is a summary of the required calculations. The lowest value represents the maximum allowable load.

Shear Failure of the bolts

$$
P_{\mathrm{S}}=n \times A_{\mathrm{B}} \times \tau_{\text {all }} \times N
$$

- $\tau_{\text {all }}$, allowable bolt shear stress depends on the the shear location; this can be along the threaded section or the smooth section of the bolt.

Bearing Failure of the Plates

$$
P_{\mathbf{P}}=d \times t \times \sigma_{-\mathrm{all}} \times N
$$

- $\sigma_{\text {-all, }}$ allowable bearing stress is 1.5 times the ultimate tensile strength of the plate material.

Gross Tensile Failure of the Plates $\quad \boldsymbol{P}_{\mathbf{G}}=\boldsymbol{b} \times \boldsymbol{t} \times \boldsymbol{\sigma}_{\mathrm{G}}$-all

- $\sigma_{\mathrm{G}}$-all, allowable gross tensile stress of the plate is $60 \%$ of the yield strength of the material

Net Tensile Failure of the Plates $\quad \boldsymbol{P}_{\mathbf{N}}=\left(\boldsymbol{b} \times \boldsymbol{t}-\mathbf{N}_{\mathbf{F}} \times \boldsymbol{d}_{\mathbf{H}} \times \boldsymbol{t}\right) \times \boldsymbol{\sigma}_{\mathbf{N} \text {-all }}$

- $\sigma_{\mathrm{N} \text {-all, }}$ allowable net tensile stress is half of the ultimate tensile strength of the plate material


## Welded joints

Welded joints are often preferred to bolted joints because they are simpler, easier to complete, relatively stronger and can provide a sealed assembly. However, they cannot be dismantled for maintenance or replacing parts.

When completing weld calculations please note the following:

- nomenclature is listed in the textbook on page 6.
- use Appendix B6 for common weld and plate size; use Appendix B5 for weld strength of common electrodes


## Weld Strength <br> $$
P_{\text {weld }}=L \times f_{\text {weld }}
$$

Gross Tensile Strength of the Plates - same as for bolted joints

## Assigned Problems

Problem 1: Two A992 steel plates are joined with two A992 steel splice plates and twelve 1 in. diameter A490 steel bolts with threads in the shear planes. Calculate the maximum load that the joint can support, in kips.


Problem 2: Two A36 steel plates form a lap joint with four 20 mm diameter A307 steel bolts. Calculate the maximum load that the joint can support, in kips.


Problem 3: Two A36 steel plates are welded with an E70 electrode.

- What is the minimum recommended weld size for this joint? [in.]
- What is the joint strength? [kips]


Problem 4: Two A36 steel plates are welded as shown with a 3/8 in. fillet weld using an E60 electrode. What is the joint strength?


Problem 5: Suggest on improvement to this chapter.

## Appendices

Appendix 1: Table for calculating centroid location and moments of inertia


Appendix 2: Beam Diagrams

| Shear <br> Force <br> Type |  | Constant <br> positive <br> slope | Constant <br> zero <br> slope | Constant <br> negative <br> slope |
| :--- | :--- | :--- | :--- | :--- |
| S.F. <br> graph | + |  |  |  |
|  | - |  |  |  |
|  |  |  |  |  |
|  |  |  |  |  |


| Shear <br> Force <br> Type |  | Positive <br> increasing <br> slope | Positive <br> decreasing <br> slope | Negative <br> increasing <br> slope | Negative <br> decreasing <br> slope |
| :--- | :--- | :--- | :--- | :--- | :--- |
| S.F. <br> graph | + |  |  |  |  |
|  | - |  |  |  |  |
|  |  |  |  |  |  |

Appendix 3: Online engineering calculators ${ }^{1}$

- SkyCiv Cloud Engineering Software
- Beam calculator
- Moment of Inertia Calculator
- Steel Beam Sizes
- Advanced Mechanical Engineering Solutions
- Calculator Edge


## Answers to Chapter Questions

## Units

Problem 1: $\sigma_{\mathrm{a}}=200 \mathrm{MPa}$; $\sigma_{\mathrm{b}}=31 \mathrm{ksi}$
Problem 2: $\mathrm{W}_{\mathrm{a}}=46 \times 10^{3} \mathrm{~N} ; \mathrm{W}_{\mathrm{b}}=10.9 \times 10^{3} \mathrm{lb}$

## Stress and Strain

Problem 1: Stress = 10.1 MPa; Strain $=0.05 \times 10^{-3}$ (hint, pipe full)
Problem 2: Load = 5655 lb .
Problem 3: It is safe (hint, $\sigma=115 \mathrm{MPa}$ )
Problem 4: $\mathrm{S}_{\mathrm{y}}=44 \mathrm{ksi}$, any material with $\mathrm{S}_{\mathrm{y}}>44$ ksi will be adequate
Problem 5: $\sigma_{\text {rod }}=127 \mathrm{ksi}, \sigma_{\text {cylinder }}=25 \mathrm{ksi}, \varepsilon_{\text {rod }}=4.2 \times 10^{-3}, \varepsilon_{\text {cylinder }}=8.9 \times 10^{-4}$

## Thermal Expansion

Problem 1: 6.34 cm below tank top
Problem 2: 468 MPa; 0.00234; 919 kN
Problem 3: 20 m ; 186.3 MPa, using $-54^{\circ} \mathrm{C}$ in the winter, $\alpha=11.7 \times 10^{-6} /{ }^{\circ} \mathrm{C}$
Problem 4: 21.45 ksi ; 180.2 kips, assuming Carbon Steel $\mathrm{E}=30 \times 10^{6} \mathrm{psi}, \alpha=6.5 \times 10^{-6} /{ }^{\circ} \mathrm{C}$
Problem 5: $\sigma_{\text {steel }}=1727$ psi; $\sigma_{\text {copper }}=7895 \mathrm{psi}$

Pressure Vessels

Problem 2: DN500 Schedule 10 pipe
Problem 3: MAWP = 690 psi
Problem 4: Does not meet specifications

## Properties of Areas

Problem 1: $7132.9 \mathrm{~cm}^{4} ; 1761.9 \mathrm{~cm}^{4}$
Problem 2: $112.85 \mathrm{in}^{4} ; 30.7 \mathrm{in}^{4}$
Problem 3: $44.32 \mathrm{~cm}^{4}$

## Beam Reactions and Diagrams

Problem 1: $\mathrm{SF}_{\max }=58.8 \mathrm{kN} ; \mathrm{BM}_{\text {max }}=44.14 \mathrm{kN} \times \mathrm{m}$
Problem 2: $\mathrm{SF}_{\text {max }}=4347 \mathrm{lbs} ; \mathrm{BM}_{\text {max }}=18263 \mathrm{lbs} \times \mathrm{ft}$
Problem 3: $\mathrm{SF}_{\text {max }}=35.98 \mathrm{kN}$; $\mathrm{BM}_{\text {max }}=83.6 \mathrm{kN} \times \mathrm{m}$
Problem 4: $\mathrm{SF}_{\max }=293.6$ kips ; $\mathrm{BM}_{\text {max }}=2420.6 \mathrm{kips} \times \mathrm{ft}$

Beam Stress due to Bending Moments

Problem 1: 72.3 MPa, safe
Problem 2: $S_{x}=766.9 \times 10^{3} \mathrm{~mm}^{3}, \mathrm{~W} 410 \times 46.1$
Problem 3: $128.6 \mathrm{lb} / \mathrm{ft}$
Problem 4: Various solutions, example: Side beams Hemlock for $S_{\min }=126$ in $^{3}$ ? choose $4^{\prime \prime} \times 16^{\prime \prime}$; deck boards $2 " \times 12$ " $\sigma_{\text {req }}=497.6$ psi ? choose Eastern White Pine

## Beam Deflection

Problem 1: 1.9 cm
Problem 2: 0.26"
Problem 3: 3.54"
Problem 4: depends on location, 37 cm @ 5.27 m from left end

Problem 1: $0.66 \times$ OD, $66 \%$ weight savings
Problem 2: 182.4 MPa, 162.97 MPa
Problem 3: 198 mm
Problem 4: 75.38 MPa, 39.95 MPa

Bolted and Welded Joints

Problem 1: TBD
Problem 2: TBD
Problem 3: TBD
Problem 4: TBD

## Authors

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Alex enjoys all that Pacific Coast offers, from skiing and camping to boating and motorcycling.

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## Revisions History

| Revisions |  |  |
| :---: | :---: | :---: |
| Revision \# | Date | Completed |
| 1 | March 2018 | - First draft complete <br> - Partially delivered the course |
| 2 | June 2018 | - Completed course delivery <br> - Editorial corrections done |
| Outstanding items: |  |  |

- Edit equations using LaTex editor
- Create more image files to replace the copyrighted images
- Inspect missing footnotes


[^0]:    Please note:

    When solving problems, if the data is given in SI units, complete the solution in SI units. Similarly for US Customary units; there is not need to switch the system of units.

